

Shear Strength of #57 Aggregates (Aggregate #A04 – TDOT Region 1)

Institute of Geotechnology

Table 1. Aggregate basic properties.

Property	Value
Specific Gravity for particles coarser than sieve #4 (ASTM-C127)	2.752
Specific Gravity for particles finer than sieve #4 (ASTM-D854)	2.721
Average Specific Gravity of Solids	2.751
Maximum Dry unit weight, Ib/ft ³ (ASTM-D4253)	105.364
Minimum Dry unit weight, Ib/ft ³ (ASTM- D4254)	88.770
Maximum void ratio (ASTM-D4254)	0.933
Minimum void ratio (ASTM-D4253)	0.629
D ₈₅ (mm)	21.50
D ₆₀ (mm)	16.30
D ₃₀ (mm)	10.35
D ₁₀ (mm)	5.80
Coefficient of uniformity, Cu	2.81
Coefficient of curvature, C _c	1.13

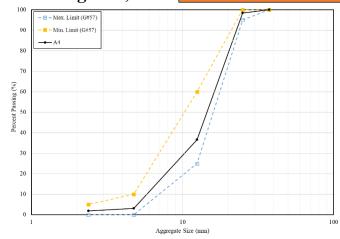


Figure 1. Gradation and size limits for A04 aggregate.

Table 2. Ch	hemical comp	osition of A04	aggregate.
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Percentage (%)
45.48
53.27
1.25
0

Figure 1 shows the gradation of the aggregate and Table 1 summarizes its physical properties. The chemical composition of the tested aggregate was identified using powder X-ray diffraction (Table 2) where multiple aggregate particles were ground into powder to a size passing US sieve #200 (75 μ m). The test was conducted at the Institute for Advanced Material and Manufacturing (IAMM), University of Tennessee-Knoxville (UTK).

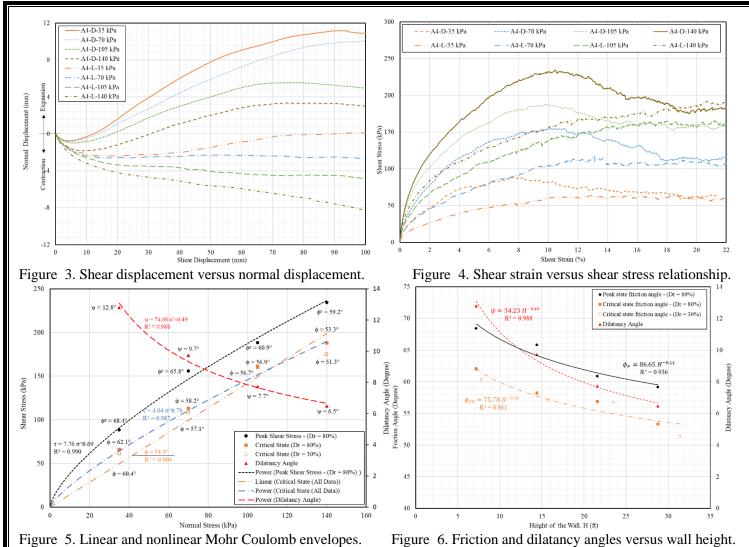
The morphology of the aggregate was measured using 3D computed tomography (CT) images for a representative sample with a diameter of 2 in. and a height of 10 in. (Figure 2). Sphericity index (I_S) refers to particles' 3D general shape regardless of angularity characteristics of corners and edges. I_S is calculated as the value of the actual volume of a particle divided by the volume of the sphere inscribed within the particle. Roundness index (I_R) is defined as the ratio of the particle's actual surface area divided by the surface area of a sphere with a size equal to the average size of the particle using its three principal axes. Form (F = shortest particle axis/ longest axis) is another shape parameter to describe granular materials. The mean values for morphology indices are $I_s = 3.046$, $I_R = 0.885$, and F = 0.460. For more details, the reader is referred to

https://alshibli.utk.edu/research/mo rphology-of-granular-materials/.

The shear strength of the aggregate was measured using a special large-scale direct shear (LSDS) apparatus at two relative densities ($D_r = 80\%$ to represent dense specimens and $D_r = 30\%$ for loose specimens). The aggregate was tested at normal stresses (σ) of 35, 70, 105, and 140 kPa (~ 5, 10, 15, and 20 psi) to represent typical stress ranges for fill aggregates behind a retaining wall. Figures 3 and 4 show the relationship between shear and normal displacements and the shear strain versus shear stress (τ) respectively. Figure 5 and Table 3 present a summary of peak state friction angles, critical state friction angles, and dilatancy angles in relation to normal stress. Figure 6 displays the same values with wall height. Table 4 lists the recommended Friction angle for the design of different wall heights.



Figure 2. CT of aggregates.



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Table 3. Summary of measured friction and dilatancy angles and the power model that relates normal stress (σ) to shear
stress (τ).	

Peak state friction angle (Power model) (ϕ_p)				Critical state friction angle (Power model) (ϕ_{cs})			Linear Mohr Coulomb for Critical state friction angle	Dilata	ncy ang Fitting (ψ)		er	
$\tau = 7.7$	$\tau=7.76~\sigma^{0.69}$, $R^{2}=0.990$			$\tau = 4.04~\sigma^{0.78}$, $R^2 = 0.987$				ψ = 74.0	0 σ -0.49	$R^{2} = 0$.988	
$\sigma = 35$ kPa	70	105	140	35	70	105	140	$\phi = 54.9^{\circ}$	35	70	105	140
68.1°	68.4° 65.8° 60.9° 59.2°	62.1°	58.2°	56.9°	53.3°	$R^2 = 0.904$	12.8°	9.7°	7.7°	6.5°		
00.4		37.2	60.4°	57.1°	56.7°	51.3°		12.0	2.1	1.1	0.5	

Table 4. Values of recommended friction and dilatancy angles for different wall heights.
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Wall Height (ft)	Recommended Friction Angle	Recommended Dilatancy Angle
< 10 ft	59	11
15 ft	57	9
20 ft	55	7
25 ft	54	7
30 ft	53	6

For more information check <u>https://alshibli.utk.edu/research/</u> or contact Professor Khalid Alshibli, Email: Alshibli@utk.edu