

## Shear Strength of #57 Aggregates (Aggregate #A08 – TDOT Region 2)

## Institute of Geotechnology

Table 1. Aggregate basic properties.

Value
2.738
2.731
2.738
104.059
89.020
0.919
0.642
20.70
15.10
8.90
5.68
2.66
0.92

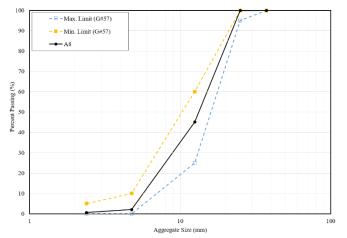


Figure 1. Gradation and size limits for A08 aggregate.

Table 2. Chemical composition of A08 aggregate.

Mineral	Percentage (%)
Calcium Carbonate (CaCo <sub>3</sub> )	51.20
Calcium Magnesium Carbonate	44.73
(Dolomite, CaMg(Co <sub>3</sub> ) <sub>2</sub> )	
Silicon Oxide (Quartz, SiO <sub>2</sub> )	4.07
Others	0

Figure 1 shows the gradation of the aggregate and Table 1 summarizes its physical properties. The chemical composition of the tested aggregate was identified using powder X-ray diffraction (Table 2) where multiple aggregate particles were ground into powder to a size passing US sieve #200 (75  $\mu m$ ). The test was conducted at the Institute for Advanced Material and Manufacturing (IAMM), University of Tennessee-Knoxville (UTK).

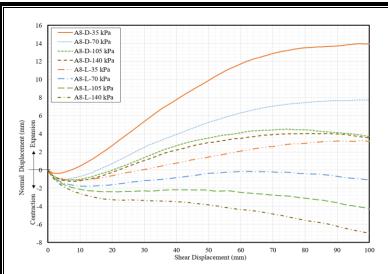
The morphology of the aggregate was measured using 3D computed tomography (CT) images for a representative sample with a diameter of 2 in. and a height of 10 in. (Figure 2). Sphericity index ( $I_S$ ) refers to particles' 3D general shape regardless of angularity characteristics of corners and edges.  $I_S$  is calculated as the value of the actual volume of a particle divided by the volume of the sphere inscribed within the particle. Roundness index (I<sub>R</sub>) is defined as the ratio of the particle's actual surface area divided by the surface area of a sphere with a size equal to the average size of the particle using its three principal axes. Form (F = shortest particle axis/ longest axis) is another shape parameter to describe granular materials. The mean values for morphology indices are  $I_S = 3.605$ ,  $I_R = 0.899$ , and F = 0.442. For more details, the reader is referred to

## https://alshibli.utk.edu/research/morphology-of-granular-materials/.

The shear strength of the aggregate was measured using a special large-scale direct shear (LSDS) apparatus at two relative densities ( $\bar{D}_r = 80\%$  to represent dense specimens and  $D_r = 30\%$  for loose specimens). The aggregate was tested at normal stresses ( $\sigma$ ) of 35, 70, 105, and 140 kPa (~ 5, 10, 15, and 20 psi) to represent typical stress ranges for fill aggregates behind a retaining wall. Figures 3 and 4 show the relationship between shear and normal displacements and the shear versus shear stress respectively. Figure 5 and Table 3 present a summary of peak state friction angles, critical state friction angles, and dilatancy angles in relation to normal stress. Figure 6 displays the same values with wall height. Table 4 lists the recommended Friction angle for the design of different wall heights.



Figure 2. CT of aggregates.



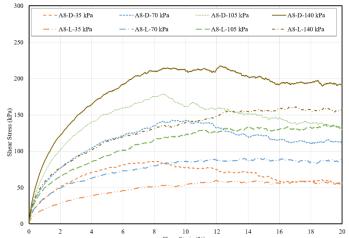
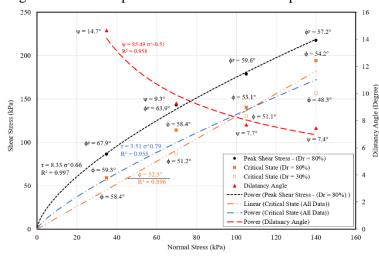


Figure 3. Shear displacement versus normal displacement.

Figure 4. Shear strain versus shear stress relationship.



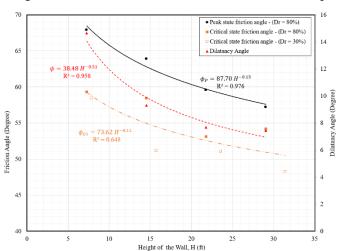


Figure 5. Linear and nonlinear Mohr Coulomb envelopes.

Figure 6. Friction and dilatancy angles versus wall height.

Table 3. Summary of measured friction and dilatancy angles and the power model that relates normal stress ( $\sigma$ ) to shear stress ( $\tau$ ).

Peak state friction angle (Power model) $(\phi_p)$				al state f (Power: (φ <sub>α</sub>	model)	ingle	Linear Mohr Coulomb for Critical state friction angle	Dilata	ncy angl Fitting (ψ)		er	
$\tau = 8.33 \ \sigma^{0.66} \ , \ R^2 = 0.997$			$\tau = 3$	.51 σ <sup>0.79</sup>	$R^2 = 0$	.895		$\psi = 85.4$	9 σ -0.51 ,	$\mathbf{R^2} = 0$	.958	
$\sigma = 35$ kPa	70	105	140	35	70	105	140	φ = 52.5°	35	70	105	140
69.7° 63.9°	63.9°   59.6°   57.2°	59.3°	58.4°	53.1°	54.2°	$R^2 = 0.896$	14.7°	9.3°	7.7°	7.4°		
	03.9	39.0	31.2	58.4°	51.2°	52.1°	48.3°		14./	9.3	7.7	7.4

Table 4. Values of recommended friction and dilatancy angles for different wall heights.

Wall Height (ft)	Recommended Friction Angle	Recommended Dilatancy Angle
< 10 ft	57	11
15 ft	54	9
20 ft	52	8
25 ft	51	7
30 ft	50	6

For more information check https://alshibli.utk.edu/research/ or contact Professor Khalid Alshibli, Email: Alshibli@utk.edu